Design process: Risk evaluation





Bruce Heinei

Design process: Risk & uncertainty evaluation



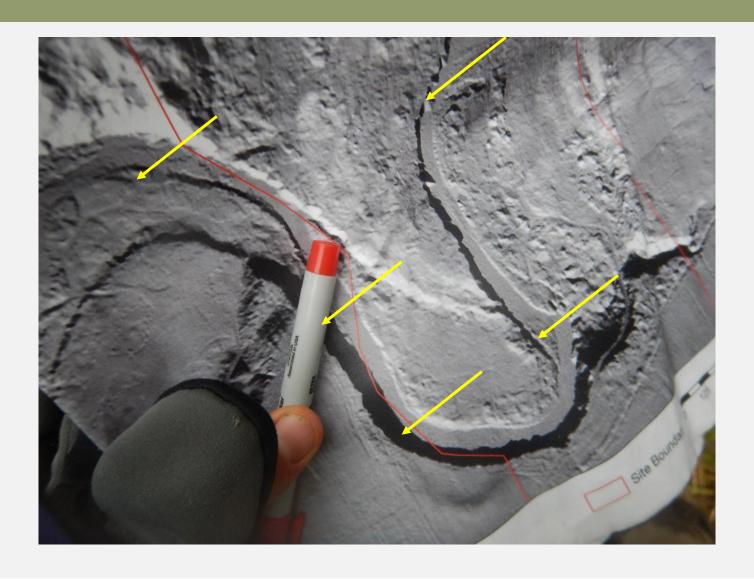
(5) A landowner whose land is used for a habitat project that is included on a habitat project list, and who has received notice from the project sponsor that the conditions of this section have been met, may not be held civilly liable for any property damages resulting from the habitat project regardless of whether or not the project was funded by the salmon recovery funding board. This subsection is subject to the following conditions:

Ellen Wohl, B

- (a) The project was designed by a licensed professional engineer (PE) or a licensed geologist (LG, LEG, or LHG) with experience in riverine restoration;
- (b) The project is designed to withstand one hundred year floods;
- (c) The project is not located within one-quarter mile of an established downstream boat launch;
- (d) The project is designed to allow adequate response time for in-river boaters to safely evade in-stream structures; and
- (e) If the project includes large wood placement, each individual root wad and each log larger than ten feet long and one foot in diameter must be visibly tagged with a unique numerical identifier that will withstand typical river conditions for at least three years.



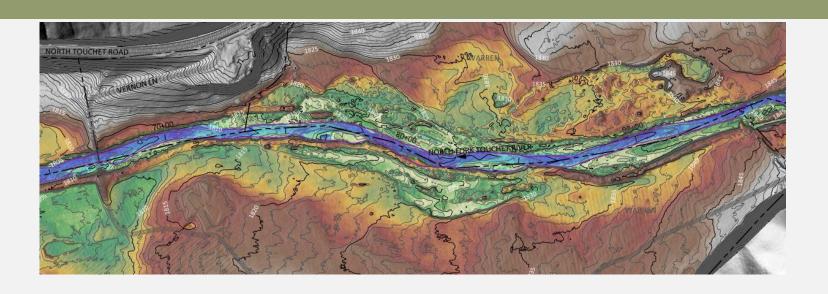
Design consideration: Site selection

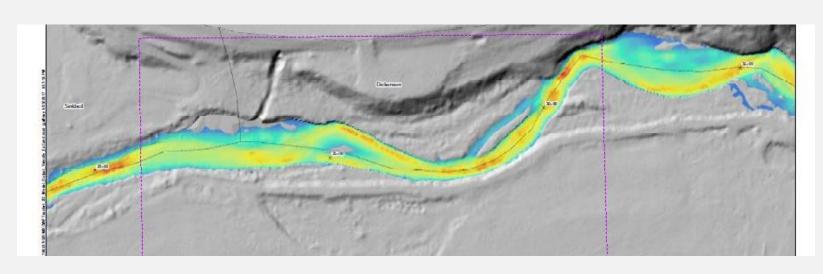


Design consideration: Site selection



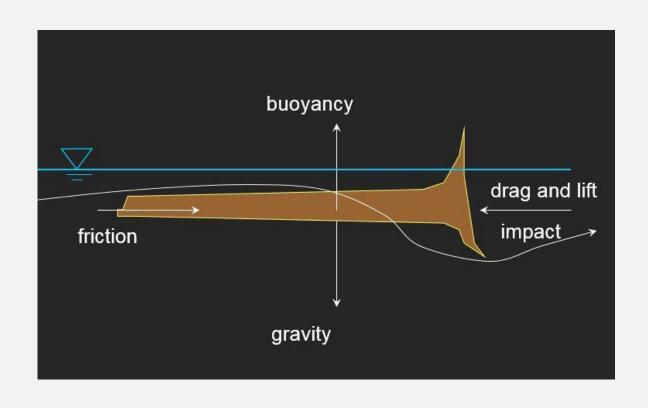
Design consideration: Site feasibility



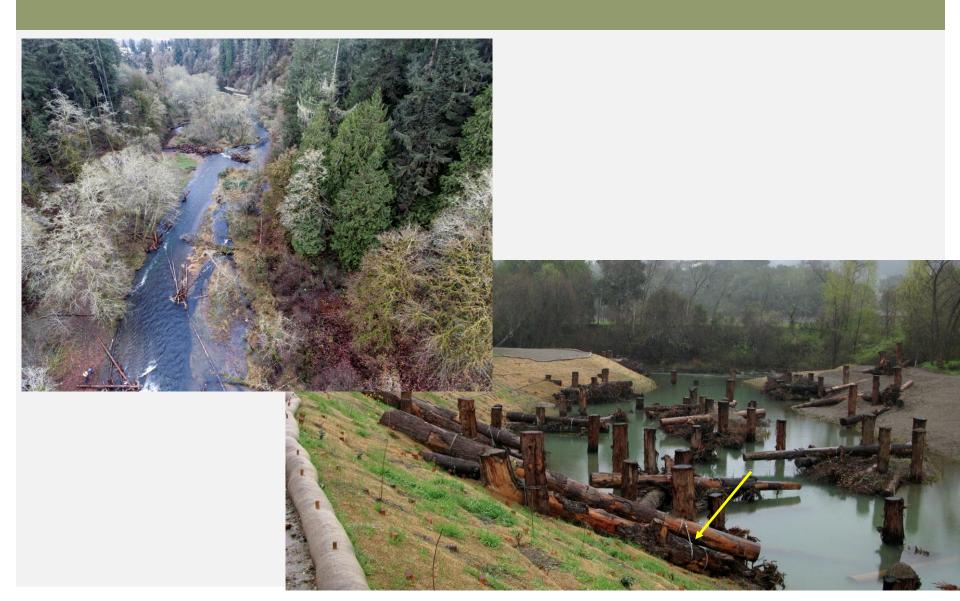


Design consideration: Stability

"Can't nobody hold me down" Puff Daddy f. Mase, 1997



Design consideration: Stability...to ballast or not?



Design consideration: Anchoring



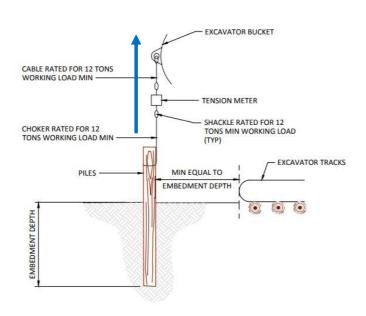
Design consideration: Vertical Pile Anchoring

"Good vibrations" – Marky Mark and the Funky Bunch, 1991

(Video)

Design consideration: Vertical Pile Anchoring

"Good vibrations" - Marky Mark and the Funky Bunch, 1991



PILES

ALL VERTICAL PILES SHALL BE INSTALLED USING VIBRASONIC PILE DRIVING EQUIPMENT. INSTALLATION BY EXCAVATION OR HAMMERING WILL NOT BE ALLOWED.

RIGGING

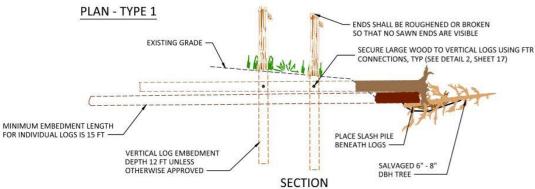
RIGGING FOR PILE TESTING SHALL CONFORM TO THE TENSION SCALE MANUFACTURER'S RECOMMENDATIONS.

CHOKERS, CABLES AND SHACKLES SHALL HAVE MINIMUM WORKING LOAD RATING OF 12 TONS. FITTINGS SHALL BE SIZED ACCORDINGLY

TESTING

TESTING OF PILES SHALL BE PERFORMED IN THE PRESENCE OF THE ENGINEER OR OTHER QUALIFIED PERSONNEL.

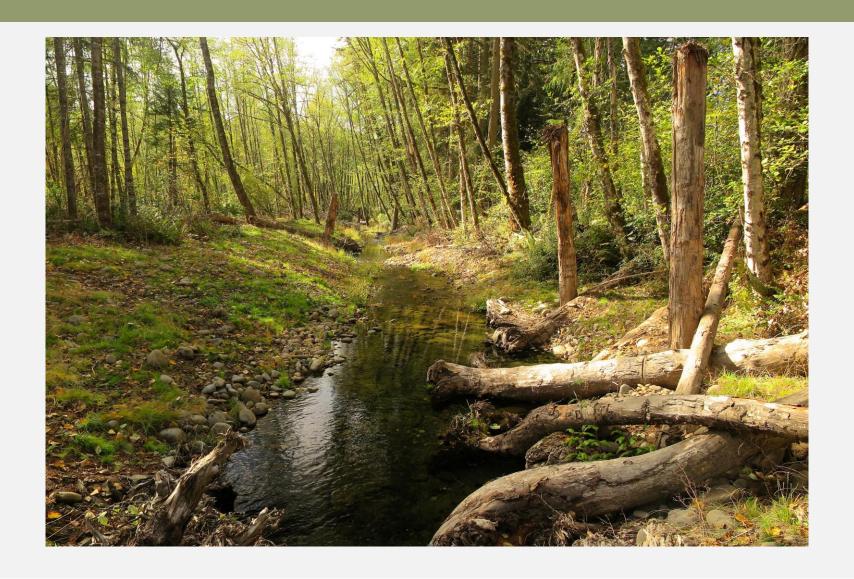
EACH PILE TEST SHALL HAVE UPWARD LOAD GRADUALLY INCREASED AND AS CLOSELY ALIGNED TO AXIS OF PILE AS POSSIBLE. RECORD THE PILE DIAMETER, EMBEDMENT DEPTH AND MAXIMUM FORCE REQUIRED TO MOVE THE PILE. UP TO



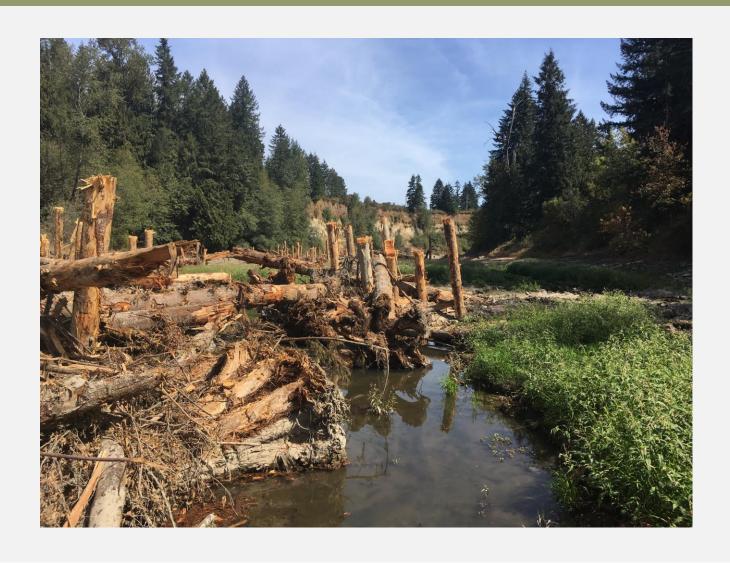




Design consideration: Size of structure



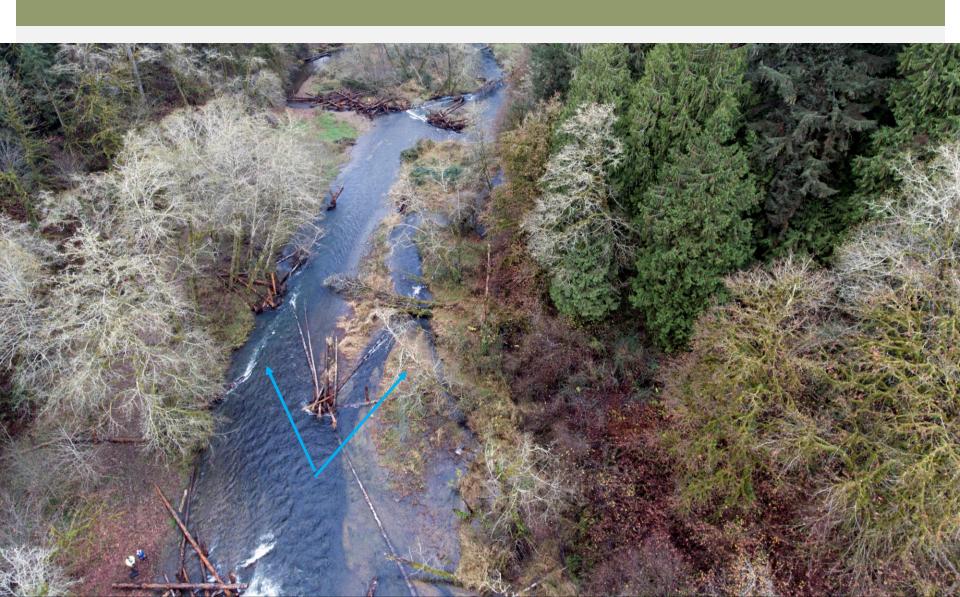
Design consideration: Size of structure



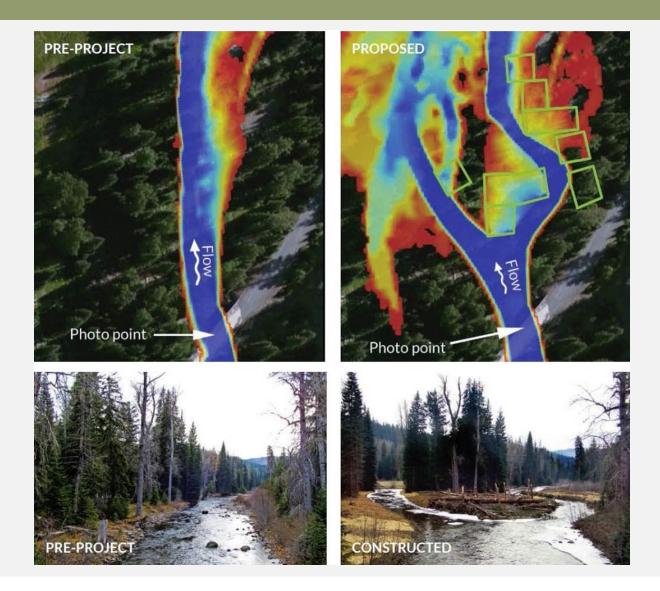
Design consideration: Size of structure



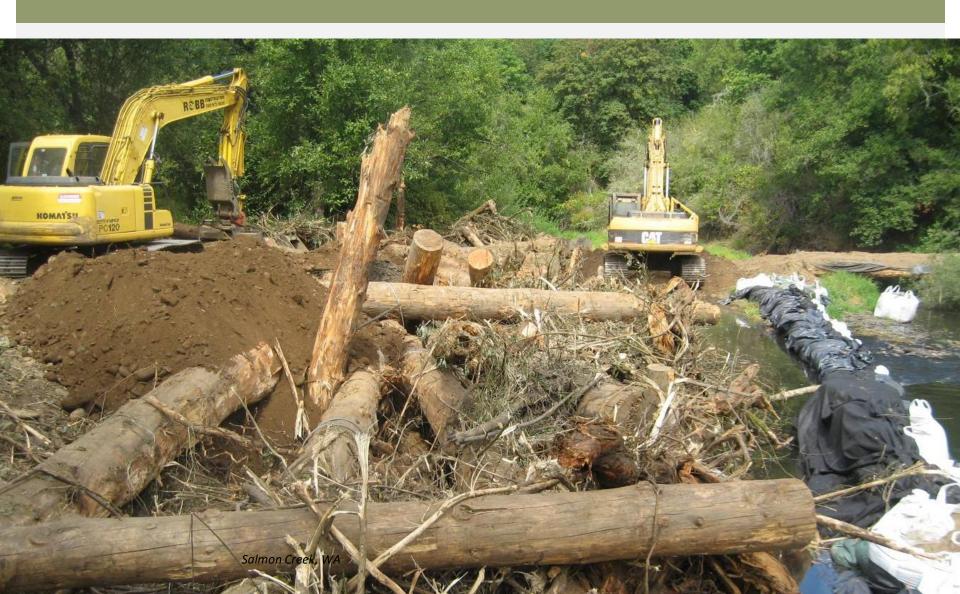
Design consideration: Placement and orientation



Design consideration: Process-promoting



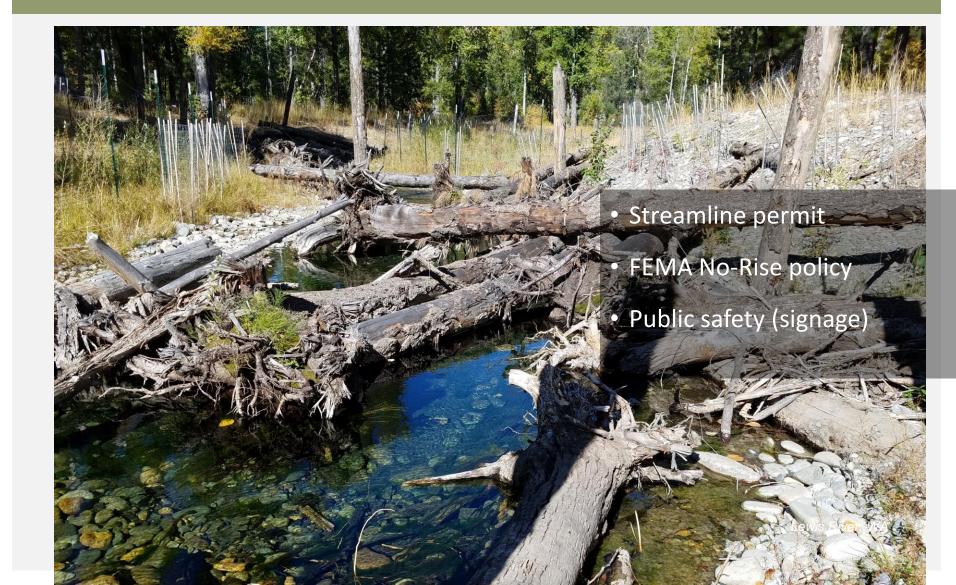
Design consideration: Brush packing



Design consideration: Materials



Project consideration: Permitting



Project consideration: Cost estimation

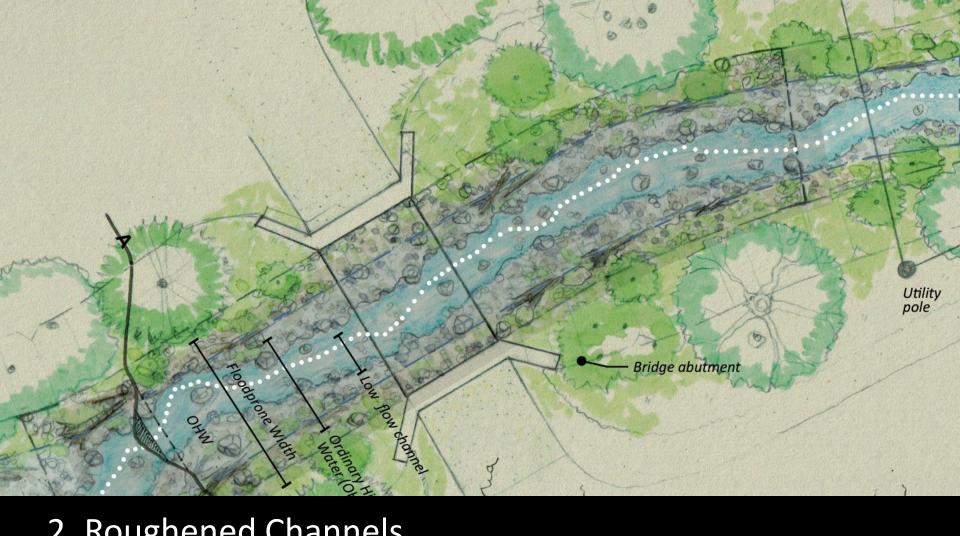


Construction considerations: Feasibility

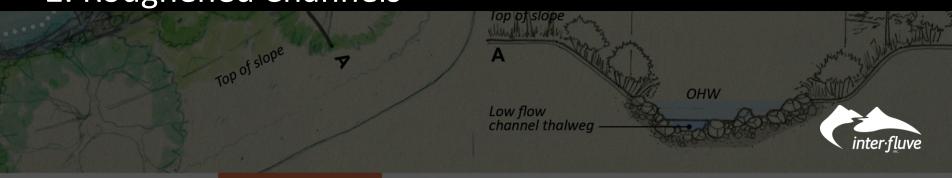


Project consideration: Monitoring and maintenance





2. Roughened Channels

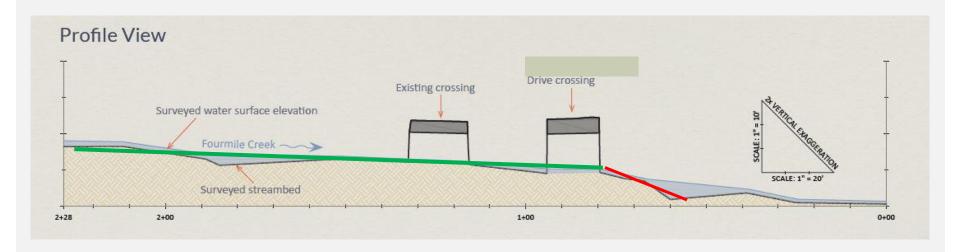




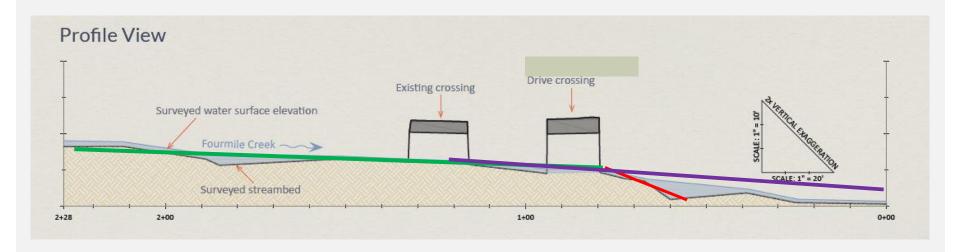
The problem

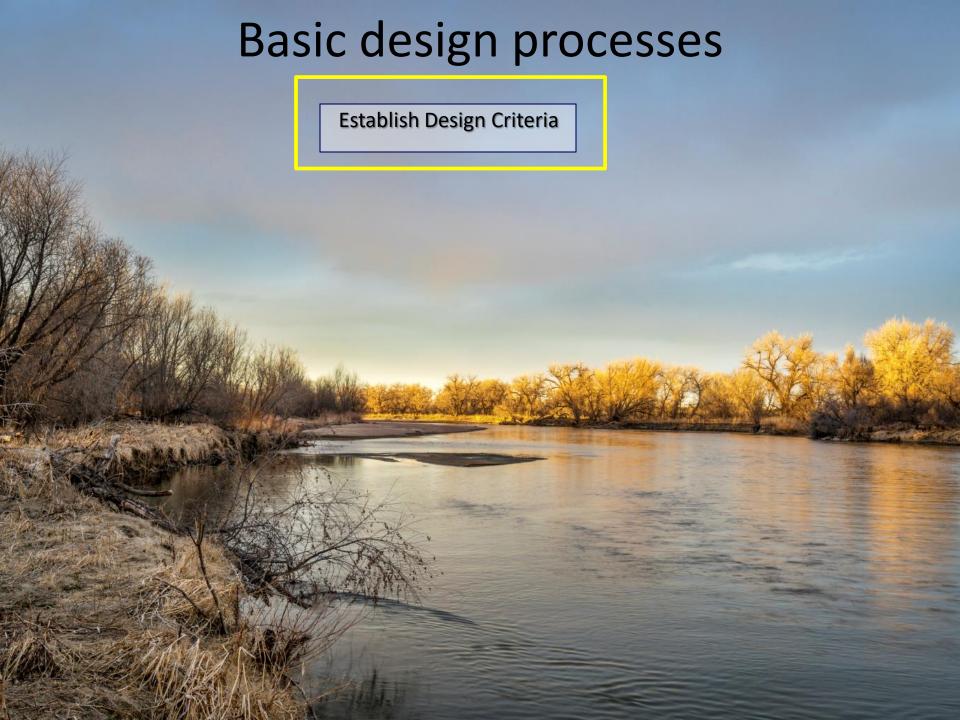


The problem



Solution: Roughened Channel







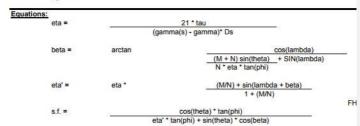
Basic design processes



Design Consideration: Substrate

Moment-Stability Rock Sizing Procedure

Ref: Pierre Julien, 1995. Erosion and Sedimentation Cambridge University Press pg 121 to 126



| SUMMARY Moment-State | pility Rock Sizing | Legend: | Entered va Result | lues | | | | |
|-------------------------|--------------------|----------------------|----------------------|-------|----------|---------|---------------|----|
| Project XS | bed shear, tau | Particle Size, D84 = | | Theta | | | Phi, angle of | |
| | psf | ft | in | H:1V | degrees | radians | degrees | ſ |
| | | | ľ | | 0 201 | | | |
| 3.0-psf | 3.00 | 1.00 | 12.0 | 3.00 | 18.43491 | 0.32175 | 41 | 0. |
| 4.0-psf | 4.00 | 1.50 | 18.0 | 3.00 | 18.43491 | 0.32175 | 41 | 0. |
| 5.0-psf | 5.00 | 1.75 | 21.0 | 3.00 | 18.43491 | 0.32175 | 41 | 0. |
| 6.0-psf | 6.00 | 2.25 | 27.0 | 3.00 | 18.43491 | 0.32175 | 41 | 0. |
| 7.0-psf | 7.00 | 2.75 | 33.0 | 3.00 | 18.43491 | 0.32175 | 41 | 0. |

USCOE Steep Slope Rock Design:

Ref: EM 1110-2-1601, Hydraulic Design of Flood Control Channels, June 30 1994, USCOE Eqn 3-5, pg D30 = (1.95 S^{0.565} (1.25 * q)^{2/3}) / (q ^{1/3})

| g D30 = | (1.95 S | 1/3) | |
|---------|----------|---------|---------|
| 10 | D30 = | 1.65 ft | 19.7 in |
| D50 = | (2.26 S | | |
| - | D50 = | 1.91 ft | 22.9 in |

Bathurst $D50 = (3.56 \text{ S}^{0.75} (1.25 \text{ q})^{2/3}) / (g^{1/3})$ D50 = 1.67 ft 20.1 in Bathurst D8 (ref. Ken Bates)

Fish-Friendly Culverts, 2004)
Where: q = 1.25 * Q/w

1.25 is flow concentration factor

s = 0.05 ft/ft Q = 639 cfs W = 15 ft

Roughened Channel Ingredients



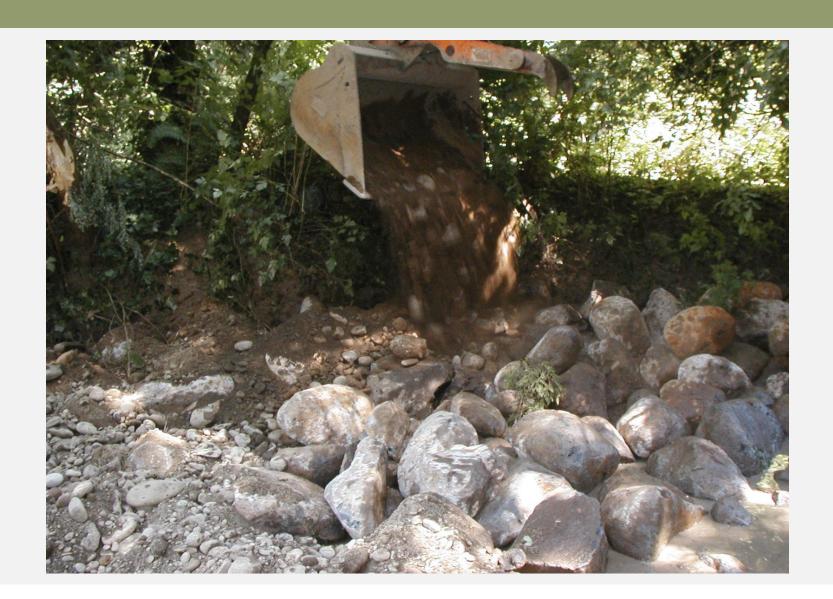
Roughened Channel Ingredients

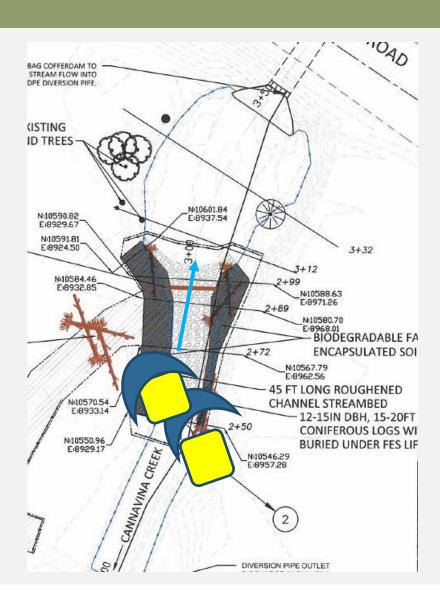


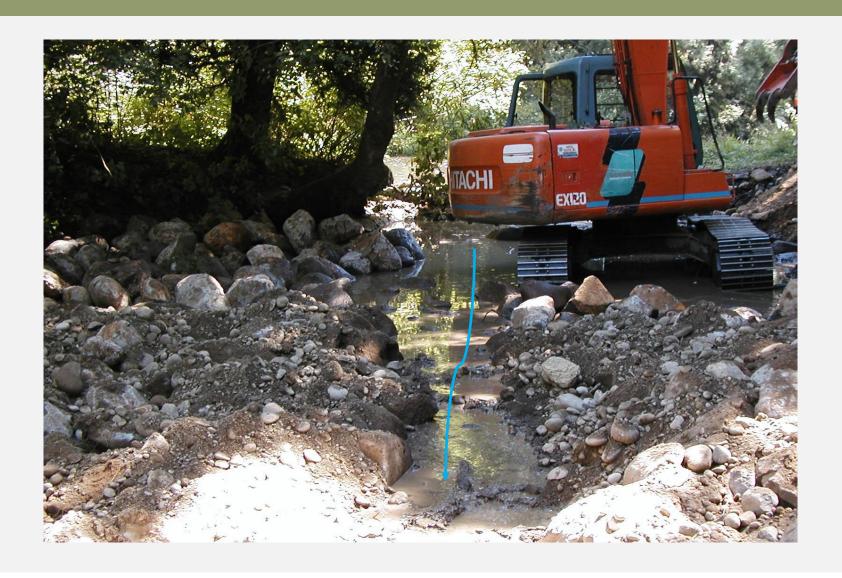
Roughened Channel Ingredients













Final Washed Surface

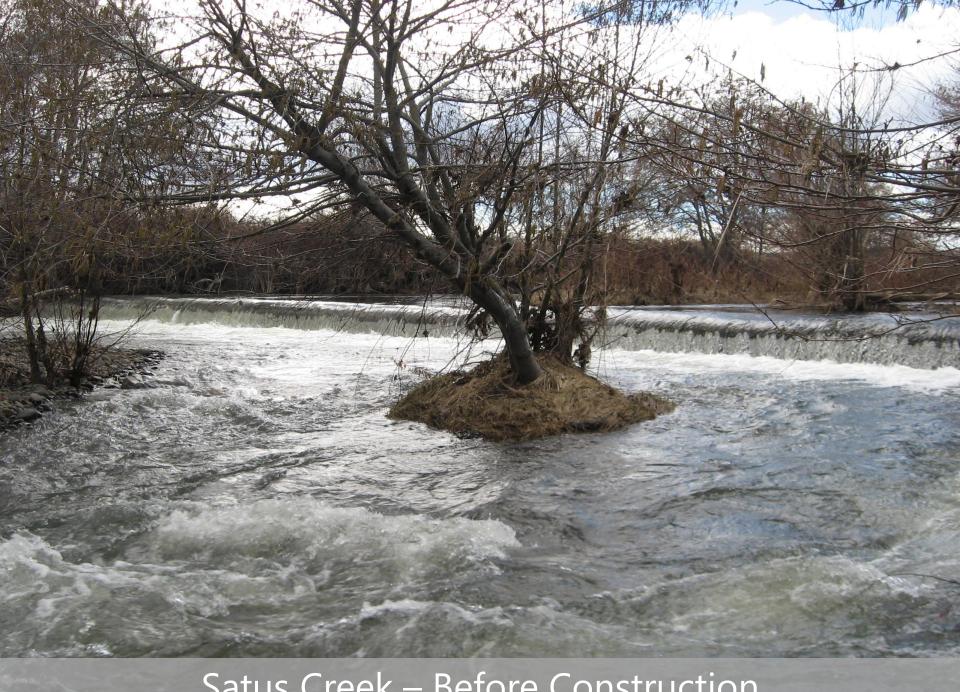




Indian Creek culvert replacement - before



Indian Creek culvert replacement - after



Satus Creek – Before Construction



Satus Creek – During Construction



Satus Creek – After Construction

